# USACE Earthquake Research & Implementation Activities

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### **Earthquakes & Dams**



 162 COE dams in high seismic areas (2 and above) subject to damage

- Most built in 1940's and 1950's with no seismic design
- Seismic design for liquefaction came into practice in the late 1970's early 1980's



# Earthquake Engineering



<u>Near failure of Lower San Fernando Dam</u> San Fernando Earthquake - 1971

### New Madrid Earthquakes, 1811-1812 (Isoseismals)



# Earthquake Effects

- Transient loading or shaking
- Changes material properties
- Settlement
- Liquefaction
- Permanent ground displacement
- Dynamic response
  - ► Each thing has it own shaking response



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# Earthquake Effects

### Liquefaction

Sand boils
Settlement
Slope failures









# Earthquake Effects

 Permanent Ground Displacement



>15 ft of thrust faulting created this waterfall and destroyed bridge



## What Levees Are

 Permanent structures (earthen, concrete/wood/steel walls, combination)

Long

- Intended for *temporary* retention of streams during high water events
- Corps responsible for over 14,000 miles of levees





# What Levees Are Not

### Dams

- They are not typically zoned to withstand long-term retention/seepage
- They do not generally include deep seepage cut-off features
- They are not typically built to withstand extreme, rare loading events (e.g., earthquakes, waves or surges)
- They are very rarely designed to withstand overtopping flow

Levees are, however, expected to protect populations and property in the same manner as dams, for brief exposure time !



# Possible Earthquake Induced Modes of Failure

- Disruption of levee by fault movement in foundation
- Loss of freeboard due to settlement or differential tectonic ground movements
- Slope failures induced by ground motions
- Sliding of levee on weak foundation materials
- Piping failure through cracks induced by ground movements
- Overtopping of levee due to seiches in waterway
- Overtopping of levee due to slides or rockfalls into waterway



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### **Vulnerability Assessment**

 Seismic vulnerability of levees and dams are similar and are evaluated as such

Liquefaction triggering analysis

Seismic slope stability analysis

Post-earthquake stability analysis

Deformation analysis, if warranted



#### Seismic Vulnerability Classes for Levees From CA Dept of Water Resources Draft Guidance Document for Urban Levees (in Review)

#### Table 6-2. Seismic Vulnerability Classes.

Amount of Deformation	Significant Damage to Internal Structures (e.g. Cutoff Walls)	Remaining Freeboard for Post Seismic Evaluation (2-Year Flood Water Surface Elevation)	Post Seismic Flood Protection Ability
<1'	No	>1'	Probably Uncompromised
1' to 3'	Possibly	>1'	Possibly Compromised
3' to 10'	Likely if existing	None	Likely Compromised
Unlimited (flow side condition)	Yes	None	Compromised



# **Current Guidance**

- ER 1110-2-1806 (Earthquake Design and Evaluation for Civil Works Projects, July 1995 - under revision)
- ER 1110-2-1156 (Safety of Dams Policy and Procedures – in final review)
- EM 1110-2-6000 (Selection of Design Earthquakes and Associated Ground Motions – in final review)
- EM 110-2-6001 (Seismic Analysis of Embankment Dams – incl. levees – ongoing)
- EM 1110-2-1913 (Design and Construction of Levees)

http://www.usace.army.mil/publications/



**Design Earthquakes and Ground Motions** 

Motions selected on performance criteria

**Performance criteria :** 

• Safety, loss of life

MDE = MCE

- Economic considerations
  - Catastrophic failure MDE < MCE
  - Little or no damage
     OBE



# **Inspection After Earthquake**

Guidance similar to that for dams

(paraphrased from USSD Guidelines for Inspection of Dams After Earthquakes, 2003)

- If an earthquake is felt at or near the dam (levee), or has been reported to occur, with:
  - ► M ≥ 4.0 w/in 25 miles,
  - M ≥ 5.0 w/in 50 miles,
  - M ≥ 6.0 w/in 75 miles,
  - M ≥ 7.0 w/in 125 miles, or
  - ▶ M ≥ 8.0 w/in 200 miles, ...immediate inspection is indicated.



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#### Isabella Dam Seismic investigation began before sPRA (very limited funding)

Initial site visit observed features apparently inconsistent with previous belief of inactivity of KCF



### **Isabella Dam** Kern Canyon Fault - initial site visits







Recent offset?

South of dam





### View South Isabella Dam

- Lava *i*s faulted, sheared
- This established that the previous primary evidence of nonactivity is invalid





Established active since 3 ma, but how recently?

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#### Isabella Dam



Aerial Photo/LiDAR comparison – Kern Canyon Fault







### Isabela Dam



Interpreted LiDAR (fault scarps)



Site had been investigated, but several scarps not seen even in field



## SEISMIC SITE CHARACTERIZATION Capable Fault

- USNRC Exhibits one or more of the following characteristics (10 CFR 100 Appendix A):
  - Movement at or near the ground surface at least once within the past 35,000 years or movement of a recurring nature within the past 500,000 years; or
  - Macroseismicity instrumentally determined with records of sufficient precision to demonstrate a direct relationship with the fault; or
  - A structural relationship to a capable fault according to characterizes (1) or (2) above, such that movement on one could be reasonably expected to be accompanied by movement on the other
- California Division of Mines and Geology
  - Surface displacement within Holocene time (about the last 10,000 years)



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## SEISMIC SITE CHARACTERIZATION Capable Fault

#### USBR

Relative displacement within the past 100,000 years

#### USACE (ETL 1110-2-301 26 Aug 1983)

- Movement at or near the ground surface at least once within the past 35,000 years
- Macroseismicity (>3.5 magnitude) instrumentally determined with records of sufficient precision to demonstrate a direct relationship with the fault
- A structural relationship to a capable fault such that movement on one could be reasonably expected to be accompanied by movement on the other
- Established patterns of microseismicity that define a fault and historic macroseismicity that can be reasonably associated with that fault



### **Triaxial Earthquake and Shock Simulator**



- TESS can simulate a wide variety of controlled, realistic motion environments
- Time and frequency domain defined tests
- Controlled six degree of freedom motion
- High-frequency, high-amplitude motion, with large payloads
- Ideal for seismic, random and shock-induced vibration testing

#### Technical Personnel

- Ghassan Al-Chaar: Structural Engineer w/experimental experience
- Steve Sweeney: Structural Engineer w/experimental experience
- Jonathan Trovillion: Materials Engineer who operates the TESS
- Jim Wilcoski: Structural Engineer w/shock & vibration experience



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### **Triaxial Earthquake and Shock Simulator**



- TESS Platform Dimensions
  - ▶ 12 ft square
  - Large test fixtures have extended this surface
- Force:
  - X-axis: 450,000 lb
  - Y-axis: 150,000 lb
  - Z-axis: 810,000 lb
- Table Accelerations w/15 kip Payload:
  - ► X-axis: 15 g
  - ► Y-axis: 4 g
  - ► Z-axis: 30 g
- Displacements:
  - ► X-axis: 2.75 in. (5.5 in. p-p)
  - Y-axis: 6.00 in. (12 in. p-p)
  - ► Z-axis: 1.375 in. (2.75 in. p-p)®

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### **Tests That Illustrate TESS Capabilities**



- Koyna Dam
- Intake Tower
- Bridge Abutment
- Cold-Formed Steel
- High-Voltage Switch
- IBM Mainframe Server
- Navy Crane Pod
- Power Transformer

Bushing



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### Seismic Testing of a 1/20 Scale Model of Koyna Dam



- Customer: ERDC-GSL
- Objective: Cast and test with sinusoidal motions a 1/20 scale model of the Koyna dam.
- Results:
  - 200 psi mix design presented unique challenges for formwork
  - Unique formwork design
  - ► Cast on the TESS



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### Seismic Testing of 1/8<sup>th</sup> Scale Model of an Intake Tower





- Customer: ERDC-GSL
- Objective: Define Failure Mode and Progression along cold-joint
- Results:
  - Scaling relations required large vertical load for gravity effects
  - 12 g, 16 Hz sinusoidal motion for failure progression
  - Documented failure development and progression along cold-joint



### Seismic Testing of Block Wall & Geotextile Bridge Abutment System





- Customer: U of WM
- Objective: Define the Failure Mode and Amplitude of Failure to Compare with Analytical Models Results:
  - Largest model tested on the TESS at 250,000 lb
  - Good control of the TESS
  - Sinusoidal motion at 1.5 and 3.0 Hz
  - Model performed well with resonant response at bearings and failure of wall
  - Measured, pressures, strains, accelerations & deformations



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# $F = ma \\ (a = g)$ g (gravity) F = ma(a = Ng) Ng (N times gravity)



THEORY

#### <u>Similitude</u>

(or, Scaling Relationships - derived from dimensional analysis)

Quantity	Full Scale (Prototype)	Centrifugal Model at N x g
Linear dimension	1	1/N
Stress (force/area)	1	1
Strain (displacement/unit length)	1	1
Density	1	1
Force	1	1/N <sup>2</sup>
Energy	1	1/N <sup>3</sup>
Displacement (distance)	1	1/N
Velocity (distance/time)	1	1
Acceleration (distance/time <sup>2</sup> )	1	Ν
Time		
Dynamic problems	1	1/N
Diffusion problems	1	1/N <sup>2</sup>
Viscous flow problems	1	1
Frequency		
Dynamic problems	1	Ν

#### **Schematic of Retaining Wall Model**



# ES-80 Performance Specifications

- Method: Servo-hydraulic multi-actuator system
- Shaking Type: Periodic or random
- Shaking Direction: One-directional
- Nominal Shaking Force: 80 kips peak dynamic force
- Max. Displacement: 0.5 inch
- Max. Velocity: 50 in./s
- Max. Payload Dimensions: 25 in. W x 49 in. L x 23 in. H (Laminar Box
- Max. Centrifugal Acceleration: 150 g



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ES – 80 Shaker Components

# Stacked rings and bearing arrangement for laminar box



# Summary

- Stresses in a centrifuge model equal those in the prototype
- Earthquakes and other dynamic loads may be replicated on models
- The ES-80 shaker provides controllable dynamic loads on the world's most powerful centrifuge platform

